

# GEOTECHNICAL INVESTIGATION

## FOR

# NSW LAND AND HOUSING CORPORATION

*7-9 Brighton Road & 21 Charles Street, Riverwood, New South Wales (BGQ69)*

*Report No: 22/1973*

*Project No: 31779/6436D-G*

*June 2022*

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DRAWING NO. 22/1973 – BOREHOLE AND PENETROMETER LOCATIONS

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## 1. INTRODUCTION

This report presents the results of a Geotechnical Investigation carried out by STS Geotechnics Pty Limited (STS) for the proposed new residential development to be constructed at 7-9 Brighton Road & 21 Charles Street, Riverwood. At the time of writing this report STS were not provided with architectural drawings for the project. The report has been prepared assuming site development will be limited to one and two storey residential buildings without basement excavation.

The purpose of the investigation was to provide information on:

- Site conditions and regional geology,
- Subsurface conditions
- Site Classification according to AS2870/AS2159 (soil reactivity),
- Foundation design parameters including foundation options, and
- Exposure classification/soil aggressiveness according to AS2870.

The investigation was undertaken in accordance with STS proposal P22-268 dated 5<sup>th</sup> May 2022.

Our scope of work did not include a contamination assessment.

## 2. NATURE OF THE INVESTIGATION

### 2.1. Fieldwork

The fieldwork consisted of drilling five (5) boreholes numbered BH1 to BH5 (inclusive), at the locations shown on attached Drawing No. 22/1973. Restricted site access dictated the borehole locations. Except for BH2 and BH5, the boreholes were drilled using a limited access track mounted Mini Christie Drilling rig, owned, and operated by STS. Because there was no access for the drilling rig, BH2 and BH5 were drilled using a hand auger. Soil strengths were assessed by carrying out a Dynamic Cone Penetrometer (DCP) tests adjacent to each borehole location.

Drilling operations were undertaken by one of STS's senior technical officers who also logged the subsurface conditions encountered.

Representative soil samples were collected from the boreholes for subsequent laboratory testing.

## 2.2. Laboratory Testing

To assess the soils for their aggressiveness, selected representative soil samples were tested to determine the following:

- pH,
- Sulphate content ( $\text{SO}_4$ ),
- Chloride content (Cl) and
- Electrical Conductivity (EC).

To assist with determining the site classification, three Atterberg Limit tests were carried out on representative samples retrieved from the site.

Detailed test reports are given in Appendix B.

## 3. GEOLOGY AND SITE CONDITIONS

The Sydney geological series map at a scale of 1:100,000 shows the site is underlain by Triassic Age Ashfield Shale of Wianamatta Group. Materials within this formation typically comprise black to dark grey shale and laminite.

At the time of the fieldwork, the site had existing dwellings with vegetation comprising grass and shrubs. The ground surface falls approximately 1.5 metres to the east.

The site is bound by Brighton Road to the east, Charles Street to the north and residential dwellings in the adjoining properties.

## 4. SUBSURFACE CONDITIONS

When assessing the subsurface conditions across a site from a limited number of boreholes, there is the possibility that variations may occur between test locations. The data derived from the site investigation programme are extrapolated across the site to form a geological model and an engineering opinion is rendered about overall subsurface conditions and their likely behaviour regarding the proposed development. The actual condition at the site may differ from those inferred, since no subsurface exploration programme, no matter how comprehensive, can reveal all subsurface details and anomalies, particularly on a site such as this where there have been previous developments.

The subsurface conditions generally consist of topsoil or fill overlying silty clays and weathered rock. Topsoil and fill are present to depths of 0.2 to 0.3 metres. Soft to firm becoming Stiff and very stiff with increasing depth, silty clays underlie the topsoil and fill to depths of 0.7 to 1.7

metres and could not be penetrated below the depths of 0.7 and 1.7 metres in BH2 and BH5. Weathered rock underlies the silty clays to the depth of drilling, 3.0 metres.

Groundwater was not observed during drilling works.

The subsurface conditions observed are recorded on the borehole logs given in Appendix A. An explanation of the terms used on the logs is also given in Appendix A. Notes relating to geotechnical reports are also attached.

## 5. GEOTECHNICAL DISCUSSION

### 5.1. Site Classification (AS2870)

The classification has been prepared in accordance with the guidelines set out in the “Residential Slabs and Footings” Code, AS2870 – 2011.

The samples collected were unsuitable for shrink swell testing. To assist with determining the site classification, three Atterberg Limits tests were carried out on representative samples retrieved from the site. Experience has shown that at times, the shrink swell index can be estimated by dividing the soil Plasticity Index (PI) by a factor of 10.

The detailed test report is attached and summarised in Table 5.1.

Table 5.1 – Atterberg Limits Test Summary

Location	Depth (m)	Material Description	Plasticity Index (%)	Inferred Shrink/Swell Index (% per $\Delta pF$ )
BH2	0.3 – 0.7	silty clay: brown with trace gravel	14	1.4
BH3	0.3 – 0.7	silty clay: brown mottled grey and orange with trace gravel	15	1.5
BH4	0.3 – 0.7	silty clay: brown with trace gravel	23	2.3

A review of the Atterberg Limits test indicates the maximum value of Plasticity Index of 23% with corresponding approximate Shrink/ Swell Index of 2.3%. Based on our experience, this value is consistent with other locations in this geological setting.

Because there are existing dwellings present, abnormal moisture conditions (AMC) prevail at the site. (Refer to Section 1.3.3 of AS2870).

Because of the AMC, the site is classified a *Problem Site (P)*. Provided the recommendations given below are adopted the site may be reclassified *Highly Reactive (H1)*.

Foundation design and construction consistent with this classification shall be adopted as specified in the above referenced standard and in accordance with the design parameters provided below.

## 5.2. Foundation Design Parameters

We do not recommend founding any structural loads within topsoil, any uncontrolled fill and soft soil.

Pad and/or strip footings founded in the natural, stiff, and very stiff silty clays may be proportioned using an allowable bearing pressure of 100 kPa. The minimum depth of founding must comply with the requirements of AS2870.

Piers founded in very stiff silty clays may be proportioned using an allowable end bearing pressure of 300 kPa, provided their depth to diameter ratio exceeds a value of 4. An allowable adhesion value of 20 kPa may be adopted for the portion of the shaft below a depth of 0.5 metres and within the very stiff clays.

If a higher load carrying capacity is required, piers founded in weathered rock may be proportioned using an allowable end bearing pressure of 700 kPa. An allowable adhesion value of 70 kPa may be adopted for the portion of the shaft in weathered rock. When piers are founded in weathered rock the adhesion within the overlying soils must be ignored.

To ensure the bearing values given can be achieved, care should be taken to ensure the base of the excavations is free of all loose material prior to concreting. To this end, it is recommended that all excavations be concreted as soon as possible, preferably immediately after excavating, cleaning, inspecting and approval. Pier excavations should not be left open overnight. The possibility of groundwater inflow needs to be considered when drilling the piers and pouring concrete.

The site is considered suitable for slab on ground construction provided due regard is given to the ground surface slope.

During foundation construction, should the subsurface conditions vary to those inferred in this report, a suitably experienced geotechnical engineer should review the design and recommendations given above to determine if any alterations are required.

### 5.3. Soil Aggressiveness

The aggressiveness or erosion potential of an environment in building materials, particularly concrete and steel is dependent on the levels of soil pH and the types of salts present, generally sulfates and chlorides. To determine the degree of aggressiveness, the test values obtained are compared to Tables 6.4.2 (C) and 6.5.2 (C) in AS2159 – 2009 Piling – Design and Installation. The test results are summarised in Table 5.2.

Table 5.2 – Soil Aggressiveness Summary Table

Sample No.	Location	Depth (m)	pH	Sulfate (mg/kg)	Chloride (mg/kg)	Electrical Conductivity (dS/m)	
						EC <sub>1:5</sub>	EC <sub>e</sub>
S1	BH1	0.4	5.3	30	<10	0.023	0.2
S2	BH3	0.5	5.4	20	<50	0.018	0.2
S3	BH5	0.4	5.2	20	<50	0.016	0.1

The soils on the site are cohesive and above groundwater. Therefore, soil conditions B are considered appropriate (AS2159).

A review of the durability aspects indicates that:

- pH : minimum value of 5.2
- SO<sub>4</sub> : maximum value of 30 mg/kg (ppm) < 5000 ppm
- Cl : maximum value of <50 mg/kg (ppm) < 5000 ppm
- EC<sub>e</sub> : maximum value of 0.2 dS/m

In accordance with AS2159-2009 the exposure classification for the onsite soils is mildly aggressive for concrete and non- aggressive for steel. In accordance with AS2870-2011 the soils are classified as A2.

Reference to DLWC (2002) “Site Investigations for Urban Salinity” indicates that EC<sub>e</sub> values of 0.1 to 0.2 dS/m are consistent with the presence of non-saline soils.

## 6. FINAL COMMENTS

During construction, should the subsurface conditions vary from those inferred above, we would be contacted to determine if any changes should be made to our recommendations. The exposed bearing surfaces for footings should be inspected by a geotechnical engineer to ensure the allowable pressure given has been achieved.

The above classification has been made assuming that all footings will bear in either natural ground or in controlled filling. Prior to the placement of any filling the existing surface should be stripped of all vegetation and topsoil.

If excavations for rainwater or detention tanks are to be made within 6 metres of the building foundations, advice should be sought regarding their effect on the foundations.

Placing absorption trenches on the high side of the property may create abnormal moisture conditions for the foundations (Refer to Section 1.3.3 of AS2870). This could have a negative effect on the foundation performance and more than likely alter the site classification provided above.

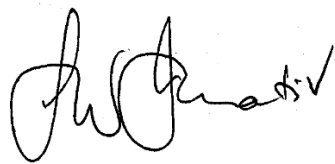
This report has been prepared assuming that no trees other than the vegetation noted will be present on the site. If future tree planting is planned, e.g., there is a landscaping plan, their effect on the foundation performance must be considered.

This report has been prepared assuming the site development will be limited to one or two storey residential buildings. The information and interpretation may not be relevant if the design proposal changes (e.g., to a five-storey building involving major cuts during the site preparation). If changes occur, we would be pleased to review the report and advise on the adequacy of the investigation.

Yours faithfully,



*Krishna Shakya*  
*Geotechnical Engineer*  
*STS Geotechnics Pty Limited*



*Laurie Ihnativ*  
*Principal Geotechnical Engineer*  
*STS Geotechnics Pty Limited*





**STS Geotechnics Pty. Ltd.**

Scale: Unknown

Date: May 2022

**Client: NSW LAND & HOUSING CORPORATION**

**GEOTECHNICAL INVESTIGATION  
7-9 BRIGHTON ROAD & 21 CHARLES STREET, RIVERWOOD  
BOREHOLE AND PENETROMETER LOCATIONS**

Project No.  
31779/6436D-G

Drawing No: 22/1973

## NOTES RELATING TO GEOTECHNICAL REPORTS

### Introduction

These notes have been provided to outline the methodology and limitations inherent in geotechnical reporting. The issues discussed are not relevant to all reports and further advice should be sought if there are any queries regarding any advice or report.

When copies of reports are made, they should be reproduced in full.

### Geotechnical Reports

Geotechnical reports are prepared by qualified personnel on the information supplied or obtained and are based on current engineering standards of interpretation and analysis.

Information may be gained from limited subsurface testing, surface observations, previous work and is supplemented by knowledge of the local geology and experience of the range of properties that may be exhibited by the materials present. For this reason, geotechnical reports should be regarded as interpretative rather than factual documents, limited to some extent by the scope of information on which they rely.

Where the report has been prepared for a specific purpose (eg. design of a three-storey building), the information and interpretation may not be appropriate if the design is changed (eg. a twenty storey building). In such cases, the report and the sufficiency of the existing work should be reviewed by STS Geotechnics Pty Limited in the light of the new proposal.

Every care is taken with the report content, however, it is not always possible to anticipate or assume responsibility for the following conditions:

- Unexpected variations in ground conditions. The potential for this depends on the amount of investigative work undertaken.
- Changes in policy or interpretation by statutory authorities.
- The actions of contractors responding to commercial pressures.

If these occur, STS Geotechnics Pty Limited would be pleased to resolve the matter through further investigation, analysis or advice.

### Unforeseen Conditions

Should conditions encountered on site differ markedly from those anticipated from the information contained in the report, STS

Geotechnics Pty Limited should be notified immediately. Early identification of site anomalies generally results in any problems being more readily resolved and allows re-interpretation and assessment of the implications for future work.

### Subsurface Information

Logs of a borehole, recovered core, test pit, excavated face or cone penetration test are an engineering and/or geological interpretation of the subsurface conditions. The reliability of the logged information depends on the drilling/testing method, sampling and/or observation spacings and the ground conditions. It is not always possible or economic to obtain continuous high quality data. It should also be recognised that the volume or material observed or tested is only a fraction of the total subsurface profile.

Interpretation of subsurface information and application to design and construction must take into consideration the spacing of the test locations, the frequency of observations and testing, and the possibility that geological boundaries may vary between observation points.

Groundwater observations and measurements outside of specially designed and constructed piezometers should be treated with care for the following reasons:

- In low permeability soils groundwater may not seep into an excavation or bore in the short time it is left open.
- A localised perched water table may not represent the true water table.
- Groundwater levels vary according to rainfall events or season.
- Some drilling and testing procedures mask or prevent groundwater inflow.

The installation of piezometers and long term monitoring of groundwater levels may be required to adequately identify groundwater conditions.

### Supply of Geotechnical Information or Tendering Purposes

It is recommended tenderers are provided with as much geological and geotechnical information that is available and that where there are uncertainties regarding the ground conditions, prospective tenders should be provided with comments discussing the range of likely conditions in addition to the investigation data.

## APPENDIX A – BOREHOLE LOGS AND EXPLANATION SHEETS

Form: I1-2

Client: NSW Land & Housing Corporation		Project / STS No. 31779/6436D-G		BOREHOLE NO.: BH 2		
Project: 7-9 Brighton Road and 21 Charles Street, Riverwood		Date: May 23, 2022				
Location: Refer to Drawing No. 22/1973		Logged: EJ Checked By: IW		Sheet 1 of 1		
W A T T A E B R L E	S A M P L E S	DEPTH (m)	DESCRIPTION OF DRILLED PRODUCT  (Soil type, colour, grain size, plasticity, minor components, observations)	S Y M B O L	CONSISTENCY (cohesive soils) or RELATIVE DENSITY (sands and gravels)	M O I S T U R E
			TOPSOIL: SILTY CLAY: dark brown, trace of gravel	CL	-	M
			SILTY CLAY: brown, trace of gravel	CL	FIRM	M
	PI	0.5			STIFF	
			HAND AUGER DISCONTINUED AT 0.7 M			
		1.0				
		1.5				
		2.0				
		2.5				
D - disturbed sample                      U - undisturbed tube sample                      B - bulk sample				Contractor: STS		
WT - level of water table or free water                      N - Standard Penetration Test (SPT)				Equipment: Hand Auger		
S - jar sample				Hole Diameter (mm): 100		
NOTES: See explanation sheets for meaning of all descriptive terms and symbols				Angle from Vertical (°): 0		
				Drill Bit: Spiral		

Client: NSW Land & Housing Corporation		Project / STS No. 31779/6436D-G		BOREHOLE NO.: BH 3		
Project: 7-9 Brighton Road and 21 Charles Street, Riverwood		Date: May 23, 2022		Sheet 1 of 1		
Location: Refer to Drawing No. 22/1973		Logged: EJ Checked By: IW				
W A T T A E B R L E	S A M P L E S	DEPTH (m)	DESCRIPTION OF DRILLED PRODUCT  (Soil type, colour, grain size, plasticity, minor components, observations)	S Y M B O L	CONSISTENCY (cohesive soils) or RELATIVE DENSITY (sands and gravels)	M O I S T U R E
	S2 @ 0.5 m		FILL: SILTY CLAY: dark grey, medium plasticity, trace of gravel	CL	-	M
			SILTY CLAY: brown mottled grey and orange, trace of shale gravel	CL	SOFT ----- STIFF	M
	PI	0.5				
			SILTY CLAY: light grey, medium plasticity, trace of gravel	CL	STIFF ----- VERY STIFF	M
		1.0				
		1.5	WEATHERED ROCK: light grey		EXTREMELY LOW STRENGTH	D
		2.0				
		2.5				
			BOREHOLE DISCONTINUED AT 3.0 M ON WEATHERED ROCK			
D - disturbed sample      U - undisturbed tube sample      B - bulk sample WT - level of water table or free water      N - Standard Penetration Test (SPT) S - jar sample				Contractor: STS Equipment: Mini Christie Hole Diameter (mm): 100		
NOTES: See explanation sheets for meaning of all descriptive terms and symbols				Angle from Vertical (°): 0 Drill Bit: Spiral		

Client: NSW Land & Housing Corporation		Project / STS No. 31779/6436D-G		BOREHOLE NO.: BH 4																												
Project: 7-9 Brighton Road and 21 Charles Street, Riverwood		Date: May 23, 2022																														
Location: Refer to Drawing No. 22/1973		Logged: EJ Checked By: IW		Sheet 1 of 1																												
W A T T E R L E	S A M P L E S	DEPTH (m)	DESCRIPTION OF DRILLED PRODUCT  (Soil type, colour, grain size, plasticity, minor components, observations)	S Y M B O L	CONSISTENCY (cohesive soils) or RELATIVE DENSITY (sands and gravels)	M O I S T U R E																										
	PI	0.5	TOPSOIL: SILTY CLAY: dark grey, trace of gravel	CL	-	M																										
			SILTY CLAY: brown, medium plasticity, trace of gravel	CL	STIFF	M																										
			SILTY CLAY: light grey, medium plasticity, trace of gravel	CL	STIFF	M																										
		1.0	1.5	WEATHERED ROCK: brown/grey		EXTREMELY LOW STRENGTH	D																									
								2.0	WEATHERED ROCK: brown		EXTREMELY LOW STRENGTH	D																				
													2.5	BOREHOLE DISCONTINUED AT 3.0 M ON WEATHERED ROCK																		

Client: NSW Land & Housing Corporation		Project / STS No. 31779/6436D-G		BOREHOLE NO.: BH 5		
Project: 7-9 Brighton Road and 21 Charles Street, Riverwood		Date: May 23, 2022				
Location: Refer to Drawing No. 22/1973		Logged: EJ Checked By: IW		Sheet 1 of 1		
W A T T A E B R L E	S A M P L E S	DEPTH (m)	DESCRIPTION OF DRILLED PRODUCT  (Soil type, colour, grain size, plasticity, minor components, observations)	S Y M B O L	CONSISTENCY (cohesive soils) or RELATIVE DENSITY (sands and gravels)	M O I S T U R E
	S3 @ 0.4 m		TOPSOIL: SILTY CLAY: dark grey, medium plasticity, trace of gravel	CL	-	M
			SILTY CLAY: brown, mottled light brown, medium plasticity, trace of gravel	CL	SOFT ----- STIFF	M
		0.5				
			SILTY CLAY: light grey mottled red, medium plasticity	CL	STIFF ----- VERY STIFF	M
		1.0				
		1.5				
		2.0	HAND AUGER DISCONTINUED AT 1.7 M			
		2.5				
D - disturbed sample                      U - undisturbed tube sample                      B - bulk sample				Contractor: STS		
WT - level of water table or free water                      N - Standard Penetration Test (SPT)				Equipment: Hand Auger		
S - jar sample				Hole Diameter (mm): 100		
NOTES: See explanation sheets for meaning of all descriptive terms and symbols				Angle from Vertical (°): 0		
				Drill Bit: Spiral		



## Dynamic Cone Penetrometer Test Report

Project: 7-9 BRIGHTON ROAD & 21 CHARLES STREET, RIVERWOOD

Project No.: 31779/6436D

Client: NSW LAND & HOUSING CORPORATION

Report No.: 22/1972

Address: 12 Darcy Street, Parramatta

Report Date: May 30, 2022

Test Method: AS 1289.6.3.2

Page: 1 of 1

Site No.	P1	P2	P3	P4	P5	
Location	Refer to Drawing No. 22/1973	Refer to Drawing No. 22/1973	Refer to Drawing No. 22/1973	Refer to Drawing No. 22/1973	Refer to Drawing No. 22/1973	
Date Tested	23/5/2022	23/5/2022	23/5/2022	23/5/2022	23/5/2022	
Starting Level	Surface Level	Surface Level	Surface Level	Surface Level	Surface Level	
Depth (m)	Penetration Resistance (blows / 150mm)					
0.00 - 0.15	2	1	1	2	1	
0.15 - 0.30	2	2	1	4	1	
0.30 - 0.45	4	3	3	2	4	
0.45 - 0.60	5	5	5	5	4	
0.60 - 0.75	4	12	4	4	3	
0.75 - 0.90	5	16	3	6	5	
0.90 - 1.05	6	22/R	10	13	22/R	
1.05 - 1.20	6		22/R	22/R		
1.20 - 1.35	22/R		Bouncing			
1.35 - 1.50						
1.50 - 1.65						
1.65 - 1.80						
1.80 - 1.95						
1.95 - 2.10						
2.10 - 2.25						
2.25 - 2.40						
2.40 - 2.55						
2.55 - 2.70						
2.70 - 2.85						
2.85 - 3.00						
3.00 - 3.15						
3.15 - 3.30						
3.30 - 3.45						
3.45 - 3.60						
3.60 - 3.75						

Remarks: \* Pre drilled prior to testing



Approved Signatory.....

Technician: EJ

Orlando Mendoza - Laboratory Manager

## E1. CLASSIFICATION OF SOILS

### E1.1 Soil Classification and the Unified System

An assessment of the site conditions usually includes an appraisal of the data available by combining values of engineering properties obtained by the site investigation with descriptions, from visual observation of the materials present on site.

The system used by STS Geotechnics Pty Ltd (STS) in the identification of soil is the Unified Soil Classification system (USC) which was developed by the US Army Corps of Engineers during World War II and has since gained international acceptance and has been adopted in its metricated form by the Standards Association of Australia.

The Australian Site Investigation Code (AS1726-1981, Appendix D) recommends that the description of a soil includes the USC group symbols which are an integral component of the system.

The soil description should contain the following information in order:

#### Soil composition

- SOIL NAME and USC classification symbol (IN BLOCK LETTERS)
- plasticity or particle characteristics
- colour
- secondary and minor constituents (name estimated proportion, plasticity or particle characteristics, colour)

#### Soil condition

- moisture condition
- consistency or density index

#### Soil structure

- structure (zoning, defects, cementing)

#### Soil origin

interpretation based on observation eg FILL, TOPSOIL, RESIDUAL, ALLUVIUM.

### E1.2 Soil Composition

- (a) Soil Name and Classification Symbol

The USC system is summarised in Figure E1.2.1. The primary division separates soil types on the basis of particle size into:

- Coarse grained soils - more than 50% of the material less than 60 mm is larger than 0.06 mm (60 µm).
- Fine grained soils - more than 50% of the material less than 60 mm is smaller than 0.06 mm (60 µm).

Initial classification is by particle size as shown in Table E1.2.1. Further classification of fine grained soils is based on plasticity.

TABLE E1.2.1 - CLASSIFICATION BY PARTICLE SIZE

NAME	SUB-DIVISION	SIZE
Clay (1)		< 2 µm
Silt (2)		2 µm to 60 µm
Sand	Fine Medium Coarse	60 µm to 200 µm 200 µm to 600 µm 600 µm to 2 mm
Gravel (3)	Fine Medium Coarse	2 mm to 6 mm 6 mm to 20 mm 20 mm to 60 mm
Cobbles (3)		60 mm to 200 mm
Boulders (3)		> 200 mm

Where a soil contains an appropriate amount of secondary material, the name includes each of the secondary components (greater than 12%) in increasing order of significance, eg sandy silty clay.

Minor components of a soil are included in the description by means of the terms "some" and "trace" as defined in Table E1.2.2.

TABLE E1.2.2 - MINOR SOIL COMPONENTS

TERM	DESCRIPTION	APPROXIMATE PROPORTION (%)
Trace	presence just detectable, little or no influence on soil properties	0-5
Some	presence easily detectable, little influence on soil properties	5-12

The USC group symbols should be included with each soil description as shown in Table E1.2.3

TABLE E1.2.3 - SOIL GROUP SYMBOLS

SOIL TYPE	PREFIX
Gravel	G
Sand	S
Silt	M
Clay	C
Organic	O
Peat	Pt

The group symbols are combined with qualifiers which indicate grading, plasticity or secondary components as shown on Table E1.2.4

TABLE E1.2.4 - SOIL GROUP QUALIFIERS

SUBGROUP	SUFFIX
Well graded	W
Poorly Graded	P
Silty	M
Clayey	C
Liquid Limit <50% - low to medium plasticity	L
Liquid Limit >50% - medium to high plasticity	H

## (b) Grading

“Well graded”	Good representation of all particle sizes from the largest to the smallest.
“Poorly graded”	One or more intermediate sizes poorly represented
“Gap graded”	One or more intermediate sizes absent
“Uniformly graded”	Essentially single size material.

## (c) Particle shape and texture

The shape and surface texture of the coarse grained particles should be described.

**Angularity** may be expressed as “rounded”, “sub-rounded”, “sub-angular” or “angular”.

Particle **form** can be “equidimensional”, “flat” or “elongate”.

**Surface texture** can be “glassy”, “smooth”, “rough”, “pitted” or “striated”.

## (d) Colour

The colour of the soil should be described in the moist condition using simple terms such as:

Black	White	Grey	Red
Brown	Orange	Yellow	Green
Blue			

These may be modified as necessary by “light” or “dark”. Borderline colours may be described as a combination of two colours, eg red-brown.

For soils that contain more than one colour terms such as:

- Speckled Very small (<10 mm dia) patches
- Mottled Irregular
- Blotched Large irregular (>75 mm dia)
- Streaked Randomly oriented streaks

## (e) Minor Components

Secondary and minor components should be individually described in a similar manner to the dominant component.

## E1.3 Soil Condition

## (a) Moisture

Soil moisture condition is described as “dry”, “moist” or “wet”.

The moisture categories are defined as:

Dry (D) - Little or no moisture evident. Soils are running. Moist (M) - Darkened in colour with cool feel. Granular soil particles tend to adhere. No free water evident upon remoulding of cohesive soils.

In addition the moisture content of cohesive soils can be estimated in relation to their liquid or plastic limit.

## (b) Consistency

Estimates of the consistency of a clay or silt soil may be made from manual examination, hand penetrometer test, SPT results or from laboratory tests to determine undrained shear or unconfined compressive strengths. The classification of consistency is defined in Table E1.3.1.

TABLE E1.3.1 - CONSISTENCY OF FINE-GRAINED SOILS

TERM	UNCONFINED STRENGTH (kPa)	FIELD IDENTIFICATION
Very Soft	<25	Easily penetrated by fist. Sample exudes between fingers when squeezed in the fist.
Soft	25 - 50	Easily moulded in fingers. Easily penetrated 50 mm by thumb.
Firm	50 - 100	Can be moulded by strong pressure in the fingers. Penetrated only with great effort.
Stiff	100 - 200	Cannot be moulded in fingers. Indented by thumb but penetrated only with great effort.
Very Stiff	200 - 400	Very tough. Difficult to cut with knife. Readily indented with thumb nail.
Hard	>400	Brittle, can just be scratched with thumb nail. Tends to break into fragments.

Unconfined compressive strength as derived by a hand penetrometer can be taken as approximately double the undrained shear strength ( $q_u = 2 c_u$ ).

## (c) Density Index

The insitu density index of granular soils can be assessed from the results of SPT or cone penetrometer tests. Density index should not be estimated visually.

TABLE E1.3.2 - DENSITY OF GRANULAR SOILS

TERM	SPT N VALUE	STATIC CONE VALUE q <sub>c</sub> (MPa)	DENSITY INDEX (%)
Very Loose	0 - 3	0 - 2	0 - 15
Loose	3 - 8	2 - 5	15 - 35
Medium Dense	8 - 25	5 - 15	35 - 65
Dense	25 - 42	15 - 20	65 - 85
Very Dense	>42	>20	>85

#### E1.4 Soil Structure

##### (a) Zoning

A sample may consist of several zones differing in colour, grain size or other properties. Terms to classify these zones are:

Layer - continuous across exposure or sample

Lens - discontinuous with lenticular shape

Pocket - irregular inclusion

Each zone should be described, their distinguishing features, and the nature of the interzone boundaries.

##### (b) Defects

Defects which are present in the sample can include:

- fissures
- roots (containing organic matter)
- tubes (hollow)
- casts (infilled)

Defects should be described giving details of dimensions and frequency. Fissure orientation, planarity, surface condition and infilling should be noted. If there is a tendency to break into blocks, block dimensions should be recorded

#### E1.5 Soil Origin

Information which may be interpretative but which may contribute to the usefulness of the material description should be included. The most common interpreted feature is the origin of the soil. The assessment of the probable origin is based on the soil material description, soil structure and its relationship to other soil and rock materials.

Common terms used are:

“Residual Soil” - Material which appears to have been derived by weathering from the underlying rock. There is no evidence of transport.

“Colluvium” - Material which appears to have been transported from its original location. The method of movement is usually the combination of gravity and erosion.

“Landslide Debris” - An extreme form of colluvium where the soil has been transported by mass movement. The material is obviously distributed and contains distinct defects related to the slope failure.

“Alluvium” - Material which has been transported essentially by water. usually associated with former stream activity.

“Fill” - Material which has been transported and placed by man. This can range from natural soils which have been

placed in a controlled manner in engineering construction to dumped waste material. A description of the constituents should include an assessment of the method of placement.

#### E1.6 Fine Grained Soils

The physical properties of fine grained soils are dominated by silts and clays.

The definition of clay and silt soils is governed by their Atterberg Limits. Clay soils are characterised by the properties of cohesion and plasticity with cohesion defines as the ability to deform without rupture. Silts exhibit cohesion but have low plasticity or are non-plastic.

The field characteristics of clay soils include:

- dry lumps have appreciable dry strength and cannot be powdered
- volume changes occur with moisture content variation
- feels smooth when moist with a greasy appearance when cut.

The field characteristics of silt soils include:

- dry lumps have negligible dry strength and can be powdered easily
- dilatancy - an increase in volume due to shearing - is indicated by the presence of a shiny film of water after a hand sample is shaken. The water disappears upon remoulding. Very fine grained sands may also exhibit dilatancy.
- low plasticity index
- feels gritty to the teeth

#### E1.7 Organic Soils

Organic soils are distinguished from other soils by their appreciable content of vegetable matter, usually derived from plant remains.

The soil usually has a distinctive smell and low bulk density.

The USC system uses the symbol Pt for partly decomposed organic material. The O symbol is combined with suffixes “O” or “H” depending on plasticity.

Where roots or root fibres are present their frequency and the depth to which they are encountered should be recorded. The presence of roots or root fibres does not necessarily mean the material is an “organic material” by classification.

Coal and lignite should be described as such and not simply as organic matter.

## APPENDIX B – LABORATORY TEST RESULTS

## Atterberg Limits and Linear Shrinkage Report

Project: 7-9 BRIGHTON ROAD & 21 CHARLES STREET, RIVERWOOD

Project No.: 31779

Client: NSW LAND & HOUSING CORPORATION

Report No.: 22/2017

Address: Level G, 12 Darcy Street, Parramatta 2150

Report Date: 7/06/2022

Test Method: AS 1289.3.1.2,3.2.1,3.4.1,2.1.1

Page: 1 OF 1

Sampling Procedure: AS 1289.1.2.1 Clause 6.5.3 - Power Auger Drilling (Not covered under NATA Scope of Accreditation)

STS / Sample No.	6463D-L / S1	6463D-L / S2	6463D-L / S3			
Sample Location	Borehole 2 Refer to Drawing No. 22/1973	Borehole 3 Refer to Drawing No. 22/1973	Borehole 4 Refer to Drawing No. 22/1973			
Material Description	Sandy Clay, orange brown grey	Sandy Gravelly Clay, orange brown grey, some fill	Gravelly Sandy Clay, orange brown grey, with some fill			
Depth (m)	0.3 - 0.7	0.3 - 0.7	0.3 - 0.7			
Sample Date	23/05/2022	23/05/2022	23/05/2022			
Sample History	Oven Dried	Oven Dried	Oven Dried			
Method of Preparation	Dry Sieved	Dry Sieved	Dry Sieved			
Liquid Limit (%)	41	40	48			
Plastic Limit (%)	27	25	25			
Plasticity Index	14	15	23			
Linear Shrinkage (%)	7.5	8.0	11			
Mould Size (mm)	250	250	250			
Crumbing	N	N	N			
Curling	N	N	N			

Remarks:

Approved Signatory.....  


Technician: DH

Orlando Mendoza - Laboratory Manager

## CERTIFICATE OF ANALYSIS

<b>Work Order</b>	<b>: ES2217998</b>	<b>Page</b>	<b>: 1 of 4</b>
<b>Client</b>	<b>: STS Geotechnics</b>	<b>Laboratory</b>	<b>: Environmental Division Sydney</b>
<b>Contact</b>	<b>: ENQUIRES STS</b>	<b>Contact</b>	<b>: Customer Services ES</b>
<b>Address</b>	<b>: Unit 14/1 Cowpasture Place Wetherill Park 2164</b>	<b>Address</b>	<b>: 277-289 Woodpark Road Smithfield NSW Australia 2164</b>
<b>Telephone</b>	<b>: ----</b>	<b>Telephone</b>	<b>: +61-2-8784 8555</b>
<b>Project</b>	<b>: 30055/30060/31779</b>	<b>Date Samples Received</b>	<b>: 24-May-2022 17:25</b>
<b>Order number</b>	<b>: 2022-0160</b>	<b>Date Analysis Commenced</b>	<b>: 26-May-2022</b>
<b>C-O-C number</b>	<b>: ----</b>	<b>Issue Date</b>	<b>: 27-May-2022 17:45</b>
<b>Sampler</b>	<b>: TS/MB/EJ/AB</b>		
<b>Site</b>	<b>: ----</b>		
<b>Quote number</b>	<b>: EN/222</b>		
<b>No. of samples received</b>	<b>: 9</b>		
<b>No. of samples analysed</b>	<b>: 9</b>		



Accreditation No. 825  
Accredited for compliance with  
ISO/IEC 17025 - Testing

This report supersedes any previous report(s) with this reference. Results apply to the sample(s) as submitted, unless the sampling was conducted by ALS. This document shall not be reproduced, except in full.

This Certificate of Analysis contains the following information:

- General Comments
- Analytical Results

**Additional information pertinent to this report will be found in the following separate attachments: Quality Control Report, QA/QC Compliance Assessment to assist with Quality Review and Sample Receipt Notification.**

### Signatories

This document has been electronically signed by the authorized signatories below. Electronic signing is carried out in compliance with procedures specified in 21 CFR Part 11.

<i>Signatories</i>	<i>Position</i>	<i>Accreditation Category</i>
Ankit Joshi	Senior Chemist - Inorganics	Sydney Inorganics, Smithfield, NSW
Ivan Taylor	Analyst	Sydney Inorganics, Smithfield, NSW



## General Comments

The analytical procedures used by ALS have been developed from established internationally recognised procedures such as those published by the USEPA, APHA, AS and NEPM. In house developed procedures are fully validated and are often at the client request.

Where moisture determination has been performed, results are reported on a dry weight basis.

Where a reported less than (<) result is higher than the LOR, this may be due to primary sample extract/digestate dilution and/or insufficient sample for analysis.

Where the LOR of a reported result differs from standard LOR, this may be due to high moisture content, insufficient sample (reduced weight employed) or matrix interference.

When sampling time information is not provided by the client, sampling dates are shown without a time component. In these instances, the time component has been assumed by the laboratory for processing purposes.

Where a result is required to meet compliance limits the associated uncertainty must be considered. Refer to the ALS Contract for details.

Key : CAS Number = CAS registry number from database maintained by Chemical Abstracts Services. The Chemical Abstracts Service is a division of the American Chemical Society.  
LOR = Limit of reporting  
^ = This result is computed from individual analyte detections at or above the level of reporting  
ø = ALS is not NATA accredited for these tests.  
~ = Indicates an estimated value.

- ED045G: LOR raised for Chloride due to sample matrix.





## Analytical Results

Sub-Matrix: SOIL (Matrix: SOIL)				Sample ID	30055/8398	30055/8404	30055/8406	30055/8407	30055/8408
Sampling date / time					23-May-2022 00:00	23-May-2022 00:00	23-May-2022 00:00	23-May-2022 00:00	23-May-2022 00:00
Compound	CAS Number	LOR	Unit		ES2217998-001	ES2217998-002	ES2217998-003	ES2217998-004	ES2217998-005
				Result	Result	Result	Result	Result	Result
<b>EA002: pH 1:5 (Soils)</b>									
pH Value	----	0.1	pH Unit		5.4	5.4	5.2	5.4	5.4
<b>EA010: Conductivity (1:5)</b>									
Electrical Conductivity @ 25°C	----	1	µS/cm		24	13	17	19	13
<b>EA055: Moisture Content (Dried @ 105-110°C)</b>									
Moisture Content	----	0.1	%		18.5	20.5	24.5	19.1	21.3
<b>ED040S : Soluble Sulfate by ICPAES</b>									
Sulfate as SO4 2-	14808-79-8	10	mg/kg		20	10	10	20	10



## Analytical Results

Sub-Matrix: SOIL (Matrix: SOIL)				Sample ID	30060/1658	31779/S1	31779/S2	31779/S3	----
Sampling date / time					23-May-2022 00:00	23-May-2022 00:00	23-May-2022 00:00	23-May-2022 00:00	----
Compound	CAS Number	LOR	Unit		ES2217998-006	ES2217998-007	ES2217998-008	ES2217998-009	-----
Result					Result	Result	Result	Result	----
<b>EA002: pH 1:5 (Soils)</b>									
pH Value	----	0.1	pH Unit		5.1	5.3	5.4	5.2	----
<b>EA010: Conductivity (1:5)</b>									
Electrical Conductivity @ 25°C	----	1	µS/cm		34	23	18	16	----
<b>EA055: Moisture Content (Dried @ 105-110°C)</b>									
Moisture Content	----	0.1	%		19.1	24.6	25.3	23.7	----
<b>ED040S : Soluble Sulfate by ICPAES</b>									
Sulfate as SO4 2-	14808-79-8	10	mg/kg		40	30	20	20	----
<b>ED045G: Chloride by Discrete Analyser</b>									
Chloride	16887-00-6	10	mg/kg		----	<10	<50	<50	----